LONG-TIME INVESTIGATION OF CEMENT PASTES WITH MICRONIZED OLD CONCRETE, INFLUENCE OF DIFFERENT TYPE OF MIXING APPROACH ON MECHANICAL PROPERTIES

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ABSTRACT. The article focuses on the impact of different type of mixing approach on resulting mechanical properties of cement composites. The monitored parameters of the mixing approach were the speed of mixing and the method of mixing the individual components of the fresh mixture. The mechanical properties of the resulting composites were the main comparative parameter. The determined mechanical properties were dynamic modulus of elasticity, dynamic shear modulus, flexural strength and compressive strength. Dynamics moduli were detected using non-destructive resonance method during the whole experiment and compressive and flexural strength were determined by destructive method for 28 and 129 days old samples. Testing samples had dimensions equal to 40 × 40 × 160 mm.

KEYWORDS: Cement composites, recycled old concrete, mixing approach, mechanical properties, long-time development.

1. INTRODUCTION

At present, the cement based composite materials (in the form of monolithic reinforced concrete, prefabricated elements, fittings etc.) present the most widespread and widely used building material. However, from the perspective of sustainable development, three basic components are needed for production – cement, sand and water. Their extraction, treatment and processing represent a burden on nature and the environment [1–3]. In particular, cement production represents 7 % of global CO2 production and large amount of NOx. Water consumption is also not desirable, it would be appropriate to reduce it. In many locations (e. g. the Middle East), there is lack of construction sand or cement, the problem is solved by transporting it from distant locations, for example, Saudi Arabia imports suitable sand from Australia and Israel imports cement from Turkey [4–6].

On the other hand, there are alternative options in the form of green composite or green concrete using alternative sources of raw materials, even secondary ones, eg. old waste concrete. The amount of waste old concrete is growing year after year and the need for its efficient use with it [7–8]. Nowadays it is relatively common to use recycled aggregate from concrete into new concrete, mostly fraction 1/16 mm [9–10]. There are several types of treatments to suitably adjust the surface of recycled aggregate to function properly in new concrete, but even these solutions have some limitations. On the other hand, fraction 0/1 mm of recycled concrete remains out of focus whether the professional or corporate (construction industry) and this fraction is estimated to be up to 20 wt. % of recycled concrete. After suitable treatment, this material can be advantageously used in cementitious composite materials, for example by mechanical-chemical activation [11–12].

The properties of the resulting composite are not only directly dependent on the grain shape which is not suitable in the case of recycled concrete but also on the size of the interfacial transition zone (ITZ). These effects have a negative influence on the resulting properties [13]. Recently, there have been studies in which the ITZ can be partially eliminated using the correct mixing approach, i.e., the correct procedure of mixing of the components of the cement composite and the mixing speed [13–15].

Unlike mentioned researches which they focus on recycled coarse aggregate, we focused on mechanical activated old concrete. This is fine aggregate with modified grain shape by using high-speed milling process [16]. The micronizing of the old concrete paste into fine particle results in uncovered unhydrated clinker minerals, which can further hydrate upon contact with water [17].

2. MATERIALS AND SAMPLES

Cement pastes samples were created to tests the mechanical properties. Cement pastes composed of 50 wt. % Portland cement CEM I 42.5R from Radotín and 50 wt. % fine recycled old concrete (ROC) has been prepared for investigating the impact of different mixing approach. The CEM I 42.5R is ordinary Portland cement with high content of C3S (approximately 75 %) and low content of C2S and C3A (< 8 %). ROC was obtained from an old construction concrete (sleep-
ers and columns with fraction 0/1 mm), which was micronized using a high-speed mill from Lavaris Ltd. (Libčice, Czech Republic). The micronized ROC with fraction 0 – 0.5 mm had mean grain size of 61.8 microns and more than 90% of all grains were less than 281 microns. Water ratio was equal to 0.35. The difference between individual samples was in the approach of mixing fresh mixtures. Two different components were mixed at 132 rpm for 240 seconds in the first step of the mixture preparation. In the second step, the remaining third component was added and the mixtures were mixed for 240 seconds at two different speeds, namely 132 rpm and 421 rpm. The mixing approaches are shown in Figure 1.

Mixtures made according to Figure 1 were cast to the rectangular molds with dimensions 40 × 40 × 160 mm. After casting, these samples were kept in the molds for 24 h at the laboratory with the temperature 22 °C. Each set consisted of 6 samples. After demolding, these samples were deposited in in a water bath at 22 ± 1 °C.

**3. EXPERIMENTAL METHODS**

Investigated mechanical parameters were developments of dynamic modulus of elasticity and dynamic shear modulus and values of flexural strength and compressive strength. The development of the dynamic modulus of elasticity and dynamic shear modulus was measured non-destructively in the first 129 days after the start of cement hydration, namely at 10th, 14th, 28th, 48th, 69th, 129th day. The flexural and compressive strength values were measured destructively on 28 and 129 day old samples.

Measurement of the dynamic modulus of elasticity was carried out by using resonance method, namely by the measurement apparatus from Brüel & Kjaer. The measurement line consisted of a measuring unit type 3560-B-X12, impact hammer type 8206, an acceleration transducer type 4519-003 53422 and software PULSE LabShop. The dynamic modulus of elasticity calculated based on the measured basic longitudinal natural frequency and basic flexural natural frequency of the samples. The dynamic shear modulus calculated based on the measured basic torsional natural frequency of the samples.

The flexural and compressive strength were determined by using destructive methods on the device FHF Strassentest. The flexural strength was determined by a three-point bending test. The testing of flexural strength was displacement controlled at a constant rate of 1 mm/min. The distance between supports for three-point bending test was equal to 100 mm. The compressive strength was determined by using uniaxial compressive test. The uniaxial compressive test was performed on the broken halves of the samples after bending test with effective dimensions of 40 × 40 × 40 mm. The testing of compressive strength was displacement controlled at a constant rate of 5 mm/min.
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**Figure 2.** Time dependence of dynamic shear modulus (with standard deviation).

**Figure 3.** Time dependence of dynamic modulus of elasticity (with standard deviation).

**Figure 4.** Flexural strength of the samples (with standard deviation).
4. RESULTS AND DISCUSSION

Results of non-destructive testing are shown in Figure 2 and Figure 3. All samples must be measured and weighed before evaluating dynamic moduli. From the measured values it was possible to determine the bulk density. All tested mixtures had approximately the same bulk density, namely 1926 ± 24 kg/m³. The results of dynamic moduli show the positive effect of the mixing speed of the fresh mixture on the dynamic modulus of elasticity and the dynamic shear modulus. Differences between the highest and lowest values of moduli for 28 days old samples were between samples CR+W_S and samples CR+W_F, namely 0.5 GPa for dynamic modulus of elasticity and 0.3 GPa for dynamic shear modulus. These differences are within the accuracy of the method. The highest increase of dynamic modulus of elasticity and dynamic shear modulus is in the first 14 days of the sample age. Furthermore, the increase in values is gradual in the next 34 days. After 48 days, there are minimal changes of dynamic moduli. The flexural strength of 28 days old samples was approximately same value on all testing samples (Figure 4). Differences were up to the size of the value standard deviation. Value of flexural strength was 4.5 MPa for 28 days old samples. For the 129 day flexural strengths, there are greater differences between the individual mixtures than for the 28 day flexural strengths. Highest value of flexural strength had sample RW+C_F, namely 6.5 ± 0.8 MPa. Lowest value of flexural strength had sample CW+R_F, namely 3.1 ± 0.7 MPa, ie more than double the difference. In contrast of it, the compressive strength values (Figure 5) were different and they have the same trend as the values of dynamic modulus of elasticity. The greatest value of compressive strength for 28 days old samples had RW+C_F, namely 40.8 ± 2.8 MPa and lowest value of compressive strength had CW+R_F, namely 28.1 ± 1.1 MPa. The difference between the compressive strength of 28 days old samples and 129 days old samples was approximately the same for all mixtures, namely 18 MPa. The increase in compressive strength between 28 and 192 days is due to a combination of two effects. The first effect is the hydration of C₂S, which is contained in CEM I 42.5R (content approximately 8 %) and the second effect is the content of unhydrated clinkers in the ROC, where this content can be up to 10 %. Value of standard deviation was approximately 5 % of compressive strength value in all tested cases.

The results show that the mixing speed has a positive effect on the resulting compressive strength, with a difference of approximately 5 MPa for slow and fast mixing of RW+C and 4 MPa for slow and fast mixing of CR+W. This is the effect of better homogenization of the fresh mixture by fast mixing. The mixture RW+C (the old concrete was first mixed with water and then with the cement) had the highest compressive strength values, namely 5 MPa higher than the mix where the old concrete was mixed with the cement at first. The mixture where the cement was mixed with water at first had the lowest compressive strength by about 5 MPa lower than in the case of cement mixing with the old concrete at first and 10 MPa lower than the mixture where the recycle was mixed with water at first. It is generally agreed that as the paste-aggregate bond strength increases (reducing of ITZ), the strength of cement composites also increases. So the process of mixing the individual components of the cement composite affects the ITZ zone between the cement matrix and the recycle grains. Results of the work are in agreement with the statement of several authors, because the size of ITZ zone depends on the approach of mixing not only in recycled coarse aggregate but also in micronized recycled old concrete. This effect can be attributed to the porous nature of the recycled aggregate, and therefore the premix process may fill some pores and cracks, resulting in an improved inter transition zone around the recycled aggregate and hence higher strength compared to the traditional mixing approach.
5. CONCLUSION

This work was focused on the influence of different type of mixing approach on mechanical properties of cement composites. The researched samples were composed of Portland cement (50 wt. %) and micronized recycled old concrete (50 wt. %). The difference between individual samples was in the approach of mixing fresh mixtures. Based on the results, it can be concluded that:

- approach of mixing fresh mixtures had influence on mechanical properties of resulting cement composites by reducing of ITZ,
- the greatest effect of type of mixing approach can be seen in the sample where water and micronized old recycled concrete were mixed in the first step and in the second step, the cement was added,
- the mixing speed has a positive effect on the resulting compressive strength, the change of mixing speed from 132 rpm to 421 rpm resulted in an increase of compressive strength of 5 MPa,
- there is an increase of compressive strength due to hydration of C2S contained in Portland clinker of reference cement and in non-hydrated clinker of micronized recycled old concrete.

In the future, the research will focus on confirming influence on approach of mixing on ITZ by using scanning electron microscope with X-ray microanalysis, namely an Energy Spectrometer.

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